




International Design Standards for CIPP Liners and their Application in the GCC Region

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Cured In Place Pipe – CIPP

- The leading trenchless sewer rehabilitation method globally
- In use for more than 40 years – first installation London 1971
- More than 12,000 km installed annually worldwide
- Diameter range from 100mm to more than 2000mm

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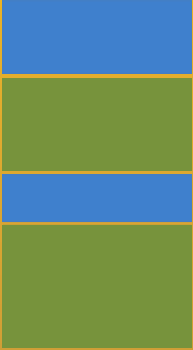
- Inversion – Air
- Inversion - Water
- Pull-in & Inflate
- Polyester Resin
- Epoxy Resin
- Vinyl Ester Resin
- Silicate Resin
- Polyester Felt Tube
- Glass Fibre Tube
- Hot Water Curing
- Steam Curing
- UV Light Curing
- LED Light Curing

96 possible combinations






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Each element can influence performance of the finished liner so must be selected carefully to meet the needs of the project in each case.

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How do we design CIPP liners?

Purpose:

- To provide structural resistance to external loads
- To stop infiltration of groundwater and/or exfiltration of sewage
- To do so for a service life of ≥ 50 years



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How do we design CIPP liners?


Design is structural

- To resist external loads safely for ≥ 50 years

Stopping infiltration/exfiltration is a property of the material




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Internationally recognised liner design methods

- **ASTM (USA): Standard F1216-16**
based on Timoshenko theory of plates and shells
(circular sewers only)
- **WRc (UK): Sewer Rehabilitation Manual**
- **DWA (Germany): Arbeitsblatt DWA-A143-2**
based on Glock theory of rigid pipes under external water pressure
- **ASTEE (France): 3R2014**
also based on Glock. For non-circular sewers



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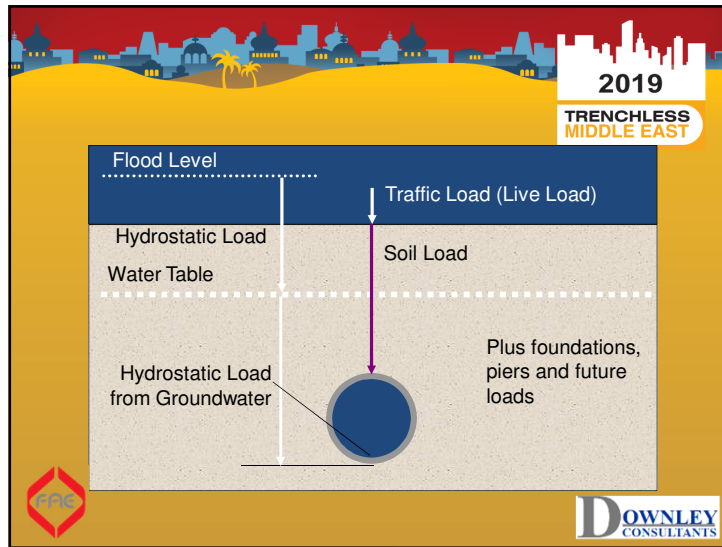
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Parameter	ASTM F1216	DWA-A143-2	Comments
Total external pressure on pipe kN/m ²	q_t	q_v	
Water pressure kN/m ²	P	p_a	
Live load kN/m ²	W_s	p_v	
Ovality %	q	Ω_v	DWA uses 2 separate ovality parameters
Ovality reduction factor	DWA-A143-2 has several further input parameters relating to geometry, joint eccentricity, and soil characteristics		
Factor of Safety			product of several partial safety between 1.10 & 1.50
Modulus of soil reaction N/mm ²	E_s'	E_2	
Long term elastic modulus N/mm ²	E_L	E_L	Modulus of liner
Liner thickness	t	S_L	
Height of soil above top of pipe m	H	h	
Soil unit weight	w	χ_s	
Height of water above pipe invert	H_w	h_w	

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How are these Parameters Measured?

- This is a very important point.
- In order to measure and verify the properties of the liners and component materials where necessary the correct test methods must be used.
- The test methods depend on the materials – different test methods may be required to test the same characteristic of different materials – for example flexural modulus (E) which is the critical characteristic for design.
- Applying the correct test, and doing it correctly, are critical in establishing characteristics for design purposes and also for product testing

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Deteriorated States

1. Leaky sewers, rigid boundary with initial shape. ASTM partially deteriorated; DWA state I
2. Broken longitudinally but structurally safe; rigid boundary of four displaced quarters. ASTM partially deteriorated; DWA state II
3. Broken and structurally unsafe; flexible boundary of four displaced quarters. ASTM fully deteriorated; DWA state III
4. Flexible boundary of granular material without capacity to transfer pressure forces – failed pipe. ASTM fully deteriorated; DWA state III

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ASTM Partially Deteriorated Design

Water Table in Ground

Ground Water Head

P = Ground water head pressure

DR = D/t

Legend: Pipe (grey), Liner (blue)

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Partially Deteriorated Design – solve 2 equations

Equation X1.1:

$$P = (2KEL)/(1-v^2) \times (1)/(DR-1)^3 \times (C/N)$$

- P = Ground water load rating for liner
- Groundwater pressure must not be more than P
- DR = D/t of liner

Equation X1.2:

$$[1.5(q/100)(1+q/100)DR^2] - [0.5(1+q/100)DR] = s/(PxN)$$

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ASTM Fully Deteriorated Design

Ground Surface

Live Load

Soil Load

Water Table

Cover

P

Existing pipe with ovality, q

D mean

Legend: Pipe (grey), Liner (blue)

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Fully Deteriorated Design – solve 4 equations

Equations X1.1 and X1.2 AND

Equation X1.3 estimates the actual pressure acting on the liner

$$q_t = (1/N)[32R_w B' E'_s C (E_t I / D^3)]^{1/2}$$

Equation X1.4

$$EI/D^3 = E/(12DR^3) \geq 0.00064$$

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Results

Example designs to ASTM F1216-16 and DWA-A143-2 with 1 glass fibre reinforced UV-cured liner:

- 4 depths to invert: 3.0, 5.0, 8.0 & 11.0m
- 4 diameters: 225, 300, 450 & 600mm
- 5% ovality
- Water table at surface
- Main/trunk road traffic loading





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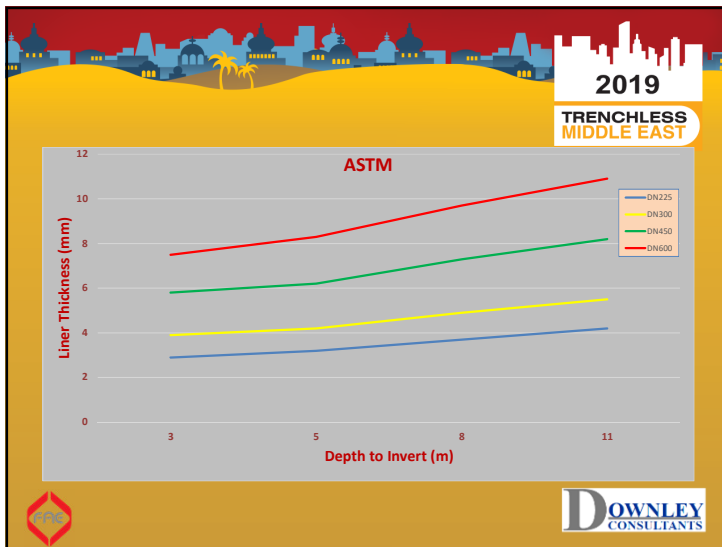
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Liner properties

- Short term flexural modulus: 10,000 N/mm²
- Long term flexural modulus: 6,800 N/mm²
- Short term flexural strength: 150 MPa
- Long term flexural strength: 105 MPa




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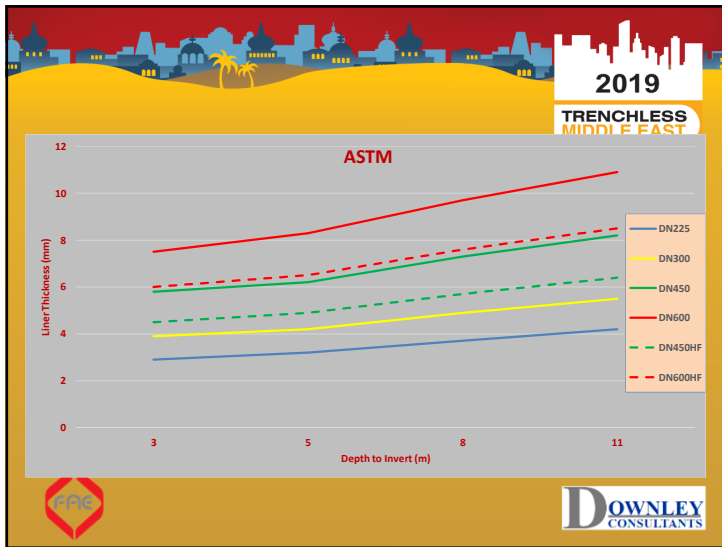
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Higher Strength Liner

- Short term flexural modulus: 17,000 N/mm² (10,000)
- Long term flexural modulus: 14,200 N/mm² (6,800)
- Short term flexural strength: 280 MPa (150)
- Long term flexural strength: 235 MPa (105)

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DWA Method State II

- Uses algorithms to solve several load cases and identify critical case.
- Calculates actual and allowable elastic deformation
- Calculates factor of safety against buckling (stability)
- Checks proof of internal and external stress

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DWA Method State III

- In addition to State II calculations:
- Calculates horizontal force from bedding reaction and lateral pressure

Horizontal soil force

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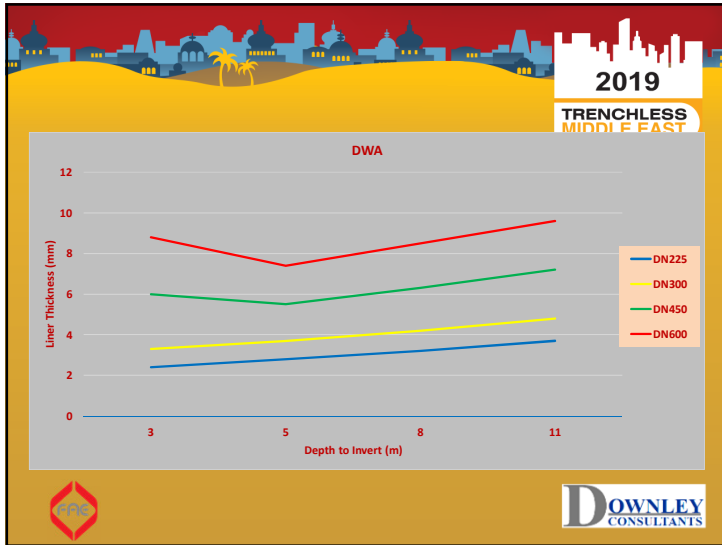
DWA Method State III

- In addition to State II calculations:
- Calculates horizontal force from bedding reaction and lateral pressure
- Calculates strain at springline of host pipe

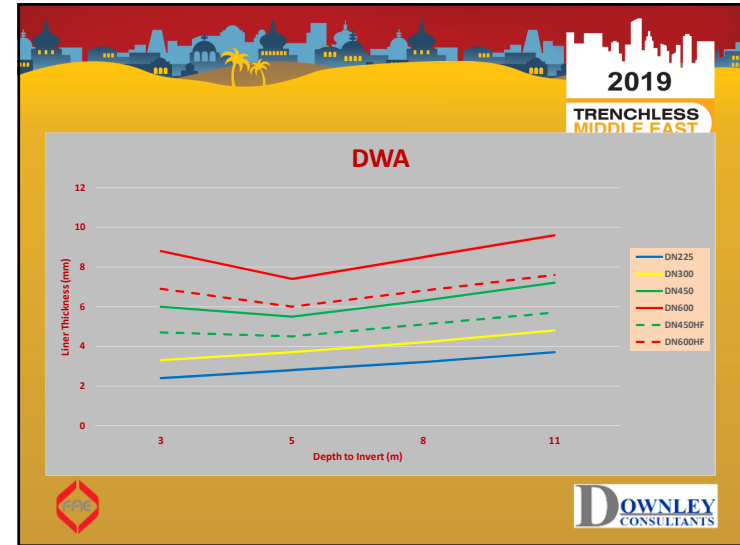
deformation figure (x 1.00)

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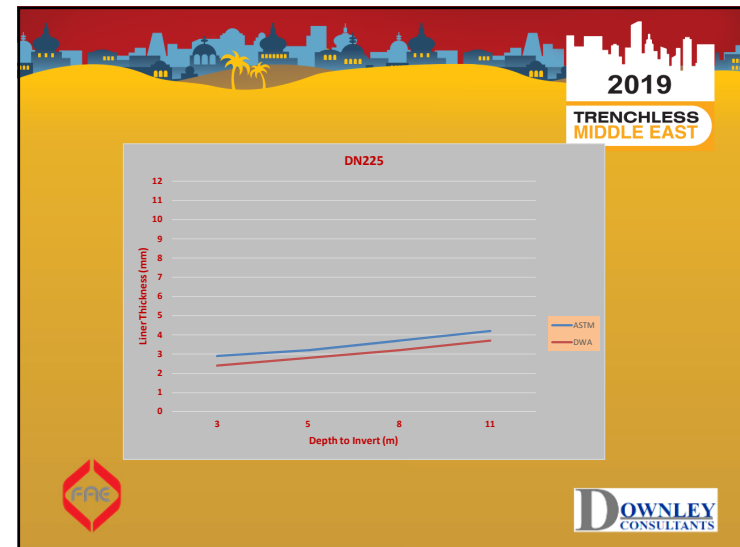
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Theoretical Differences

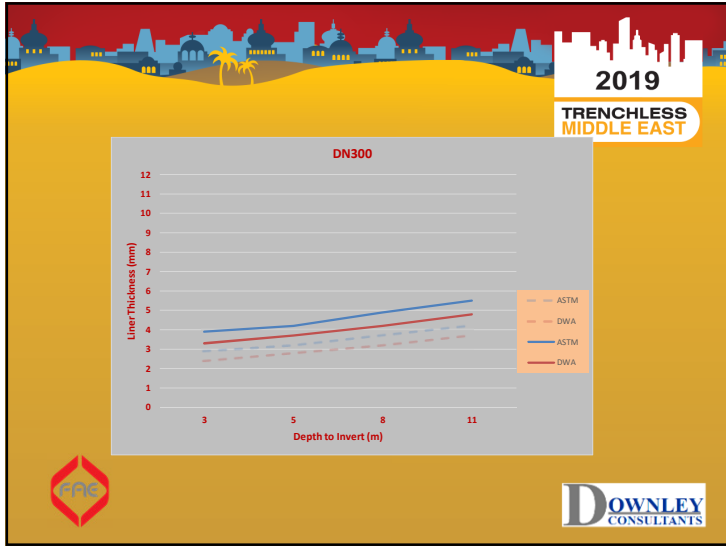
Parameter	ASTM F1216	DWA-A143-2
Basic theory	Timoshenko	Glock
Critical water pressure	Free ring supported by host pipe (enhancement factor 7)	Ring in a cavity
Critical live load pressure	Fully embedded liner	Ring supported by four quarters of host pipe embedded in soil

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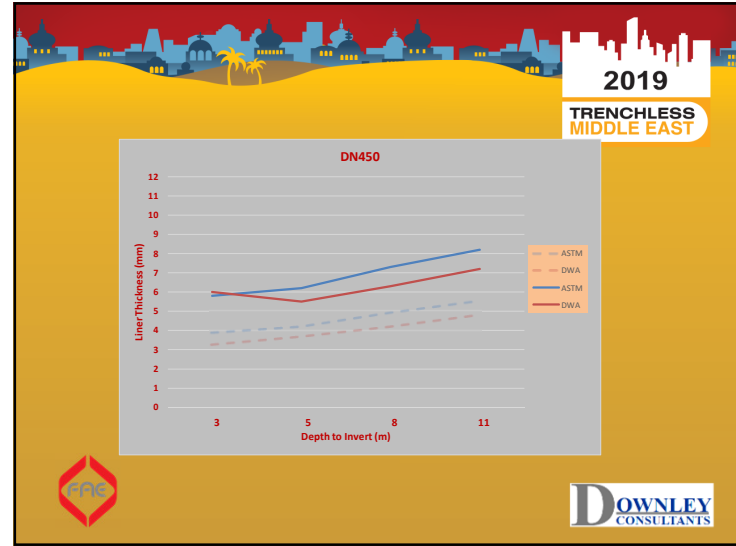
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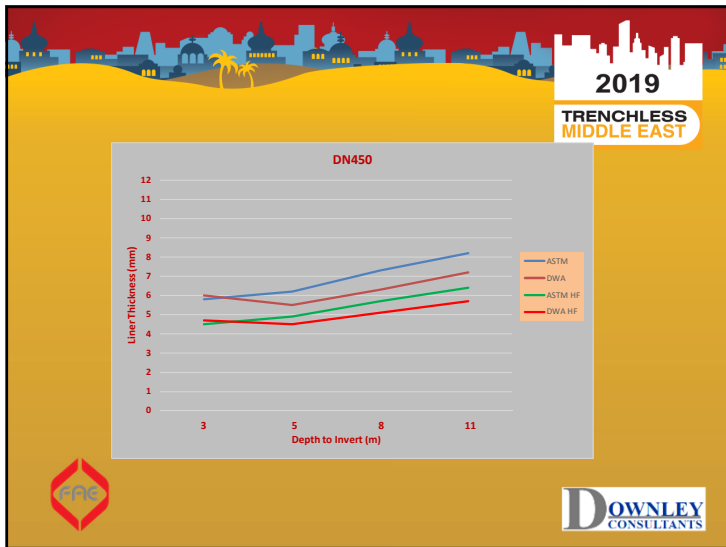
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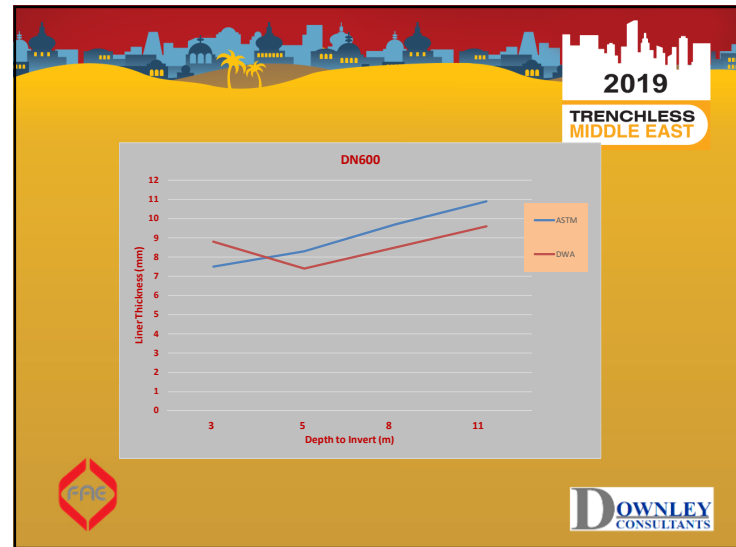
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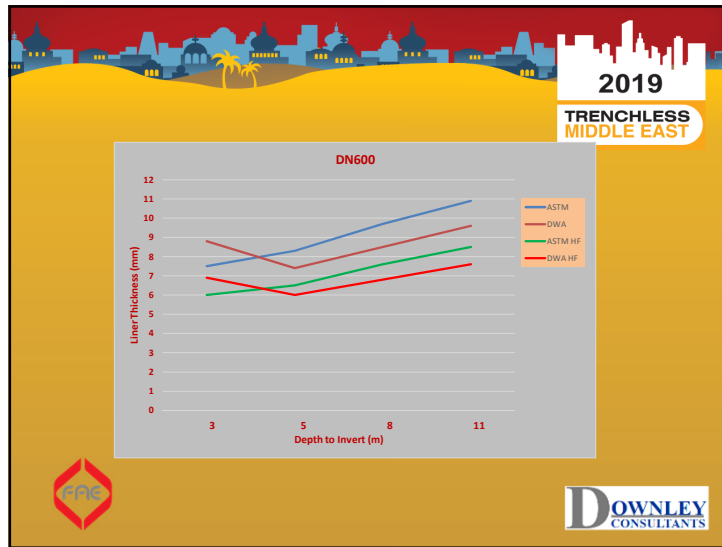
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CONCLUSIONS

- Both design methods are based on well-established structural theory and the basic principles underlying the design methods differ
- The results are similar in most cases - the DWA method generally results in slightly thinner liners for a given load condition
- The ASTM method is simpler and has higher inbuilt safety factor (which explains the thicker liners resulting from this method)
- Both design methods result in safe liner designs
- DWA design is better suited to shallow sewers with high live loading

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Thank you for your attention

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